# Shredded Wheat Factory Drainage Strategy

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Client Name: Plutus Estates (WGC) and Metropolitan Housing Trust

Client Address: One York Road Uxbridge UB8 1RN

Site Address: Bridge Road End Welwyn Garden City AL7 3UQ





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Drainage Strategy

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Author	Signature	Date
Adam Smith BSc(Hons) Infrastructure Engineer		28/02/2018

Reviewed	Signature	Date
Renee Upton BEng(Hons) Senior Infrastructure Engineer		28/02/2018



Drainage Strategy

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Drainage Strategy

# 1.0 Introduction

### 1.1 Project Background

Curtins Consulting Limited has been appointed by Metropolitan Housing Trust to prepare a Drainage Strategy report for the proposed residential development at the old Shredded Wheat Factory near Bridge Road End, Welwyn Garden City AL7 3UQ.

Proposals contained or forming part of this report represent the design intent and may be subject to alteration or adjustment in completing the detailed design for this project. Where such adjustments are undertaken as part of the detailed design, and are deemed a material derivation from the intent contained in this document, prior approval shall be obtained from the relevant authority in advance of commencing such works.

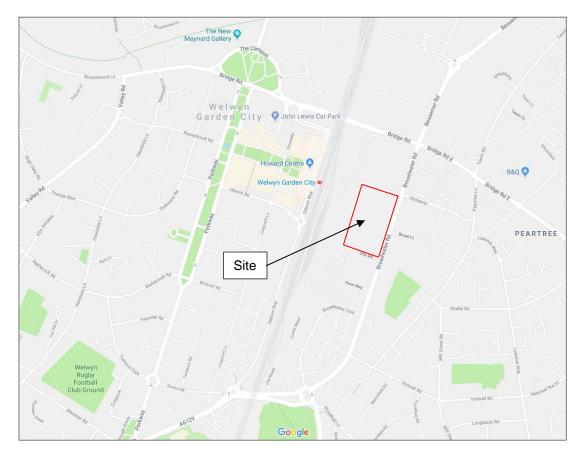


Figure 1 – Site location Plan



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# 2.0 Flood Risk Assessment

A Flood Risk Assessment was previously carried out in February 2015 by ICIS Limited (report reference: 100007) to obtain outline planning permission, which demonstrated the site is located within Flood Zone 1 (low risk).

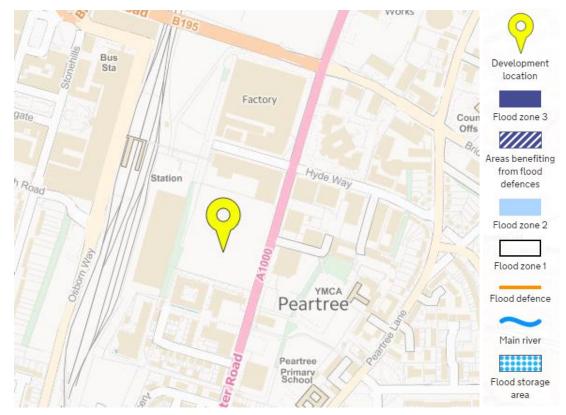


Figure 2 – Environment Agency Flood Map

Other sources of flooding including pluvial (surface water), sewer, reservoirs and groundwater were assessed and considered to be low.

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# 3.0 Drainage Strategy

### 3.1 Existing Drainage

The topographical survey carried out by Laser Surveys in October 2014 (drawing reference: L 6080 Rev 1) demonstrates the slope across the site is generally flat, with levels ranging from 84.17 – 85.60 AOD, sloping from north-west to south-east.

The topographical survey is contained within Appendix A.

Thames Water (TW) sewer records indicate two parallel surface water sewers are located within Broadwater Road to the east of the site, and discharge to the north. The first surface water sewer upsizes from 825 to 900mm diameter and is approximately five metres deep. The second surface water sewer upsizes from 225 to 300mm diameter and is approximately four metres deep.

A 225mm foul sewer is also located within Broadwater Road and discharges to the north, and is approximately four metres deep.

TW sewer records are contained within Appendix B.

A ground-penetrating radar (GPR) survey was also carried out by MK Surveys in January 2014 (project number ref: 19040) and demonstrates the site was previously served by a separate foul and surface water network, which has largely been abandoned.

It is noted however, several foul and surface water manholes immediately adjacent to Broadwater Road, along with their downstream connections to the public sewer, are still present. It is therefore proposed to carry out a CCTV survey of these drain runs, and re-use them for the proposed drainage network, if shown to be feasible.

The GPR survey is contained with Appendix C.

### 3.2 Proposed Drainage Design

A new drainage system has been designed for the proposed development, with separate surface and foul water networks, which both connect into the public sewer to the east in Broadwater Road (refer to Appendix D for the Proposed Drainage Layout).

A Pre- Development enquiry has been submitted to Thames Water to determine the capacity, condition, etc. of the public sewer and allowable discharge rate. At the time of writing this report, Curtins are awaiting a response from TW.

Drainage Strategy



### 3.3 Proposed Surface Water Network

Surface water from building roofs will be conveyed to ground floor level through a series of rainwater pipes, and connect to the below ground network. Runoff in hardstanding external areas (outside the proposed building footprint) will collect to gullies, channels and permeable paving which will be located in accordance with proposed levels. Exact locations will be coordinated with the landscape architect.

It is proposed to provide three separate surface water networks across the site, which will all discharge separately into the public sewer within Broadwater Road. Existing connections have been identified on site and it is proposed to re-use them, subject to the findings of the CCTV drainage survey.

Proposed surface water runoff from site will be restricted to Greenfield rates (5 I/s/Ha) which equates to 19.85 I/s for the whole site, in line with the Strategic Flood Risk Assessment (SFRA) for Welwyn Hatfield Borough Council produced by Scott Wilson (May 2009). This is a significant reduction in the surface water regime, as the existing surface water runoff rate from site to the public sewer has been calculated to be 143 I/s (refer to Appendix E).

The required surface water attenuation storage volume has been sized preliminarily using Micro Drainage Source Control (Appendix E), to attenuate up to the 1 in 100 year rainfall event, with an allowance for climate change (40%). The Micro Drainage preliminary estimates indicate approximately 2674m<sup>3</sup> of attenuation is required. It is noted this attenuation volume will be reduced in due course, subject to a more detailed model using Micro Drainage Network, which will account for grassed landscaping areas, green roofs, and sustainable urban drainage systems (SuDS) proposed across site.

It is proposed that surface water storage will be distributed within ponds, permeable paving course graded aggregate build-ups and geocellular storage tanks (subject to detailed design).





Figure 3 – Proposed Drainage Layout

### 3.4 Proposed Foul Water Network

A foul water network will be provided in areas adjacent to the building, which will collect flows from soil vent pipes, stub stacks, foul gullies, etc. Foul manholes and drains have been located where these vertical drain points are likely to be, subject to the design of the M&E engineer.

The foul network will discharge to the public foul sewer within Broadwater Road to the east of the site, and the proposed network will be provided at a later detailed design stage.





# 4.0 Sustainable Drainage Strategy (SuDS)

### 4.1 SuDS Hierarchy

Developments should utilise sustainable drainage systems (SuDS) unless there are practical reasons for not doing so. The design should aim to reduce run-off rates and ensure that run-off is managed as close to its source as possible.

The SFRA for Welwyn Hatfield Borough Council in May 2009, which outlines a sustainable drainage hierarchy, has been considered in the design of the site's surface water system.

Figure 4 provides rationale for selecting/neglecting certain techniques for this development.



SUDS	Site Specific Analysis
Technique	
Living Roofs/Areas	Green roofs are currently proposed for the residential blocks, and the exact areas will be determined at the detailed design phase.
Basins and Ponds	There may be an opportunity to incorporate a large dry (detention) pond within the central area of the site, along with a wet (retention) pond at the north-east corner of the site. This will be subject to coordination with the landscape architect.
Filter Strips and Swales	Due to a large area being required for public and play areas for children, open flow channels are not considered to be feasible for this site.
Infiltration Devices	Although the site is underlaid by chalk, Curtins geo-environmental team has advised discharging to the ground would undermine the foundations for the high rise residential blocks, and doing so would be high risk for this site.
	There may be some opportunities to discharge runoff from footpaths via permeable paving into the ground, and will be reviewed at the detailed design stage.
Permeable Surfaces	It is proposed to include permeable surfaces within the parking bays, footpaths and play areas across the site. The exact areas will be subject to coordination with Curtins geo-environmental team as the design progresses.
Tanked Systems	The remainder of surface water from the site will be attenuated in cellular storage tanks, whilst being slowly discharged into the public sewer. Whilst discharge into a sewer is the least favourable option in the sustainable drainage hierarchy, the lack of an alternative public surface water system or nearby watercourse means there is no other option. This attenuation volume has been based on proposed discharge rates
	Technique Living Roofs/Areas Basins and Ponds Filter Strips and Swales Infiltration Devices Permeable Surfaces Tanked

### Figure 4: SuDS Hierarchy

Drainage Strategy



## 5.0 Summary

Three separate surface water networks are proposed across the site, which will discharge separately into the public sewer within Broadwater Road.

A GPR survey identified several existing foul and surface water manholes immediately adjacent to Broadwater Road, along with their downstream connections to the public sewer. It is therefore proposed to carry out a CCTV survey of these drain runs, and re-use them for the proposed drainage network, if shown to be feasible.

Surface water runoff from site will be restricted to Greenfield rates (5 l/s/Ha) which equates to 19.85 l/s for the whole site, in line with the SFRA for Welwyn Hatfield Borough Council. This is a significant reduction in the surface water regime, as the existing surface water runoff rate from site to the public sewer has been calculated to be 143 l/s.

The required surface water attenuation storage volume for the whole site has been sized using Micro Drainage Source Control to attenuate up to the 1 in 100 year rainfall event, with an allowance for climate change (40%).

The Micro Drainage preliminary estimates indicate approximately 2674m<sup>3</sup> of attenuation is required. It is noted this attenuation volume will be reduced in due course, subject to a more detailed model using Micro Drainage Network, which will account for grassed landscaping areas, green roofs, and SuDS proposed across site.

It is proposed that surface water storage will be distributed within ponds, permeable paving course graded aggregate build-ups and geocellular storage tanks (subject to detailed design).



Appendix A – Topographical Survey



Appendix B – Thames Water Sewer Records



Appendix C – GPR Survey by MK Surveys



Appendix D – Proposed Drainage Layout

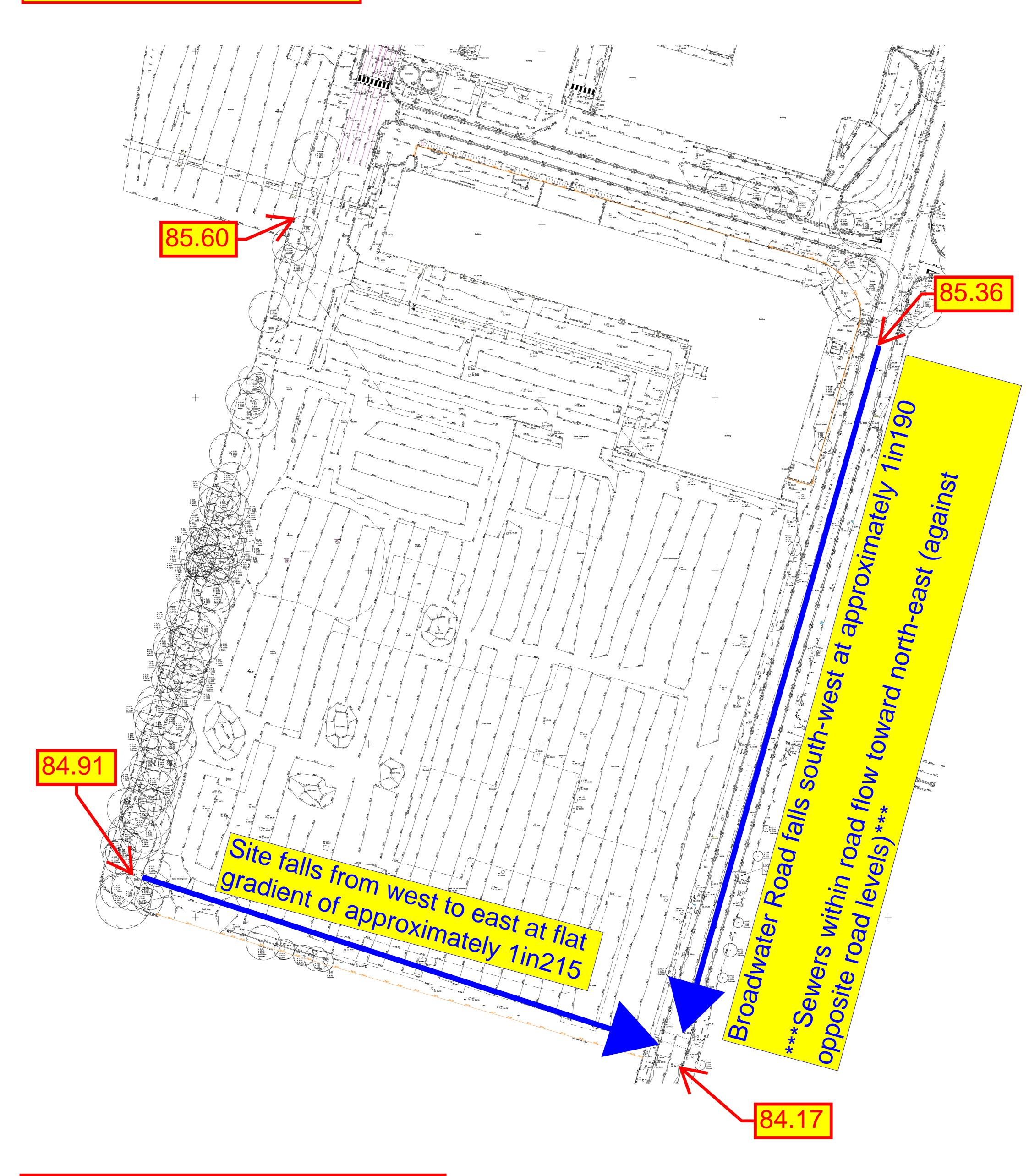


# Appendix E – Existing Surface Water Rates and Micro Drainage Calculations



Appendix A – Topographical Survey



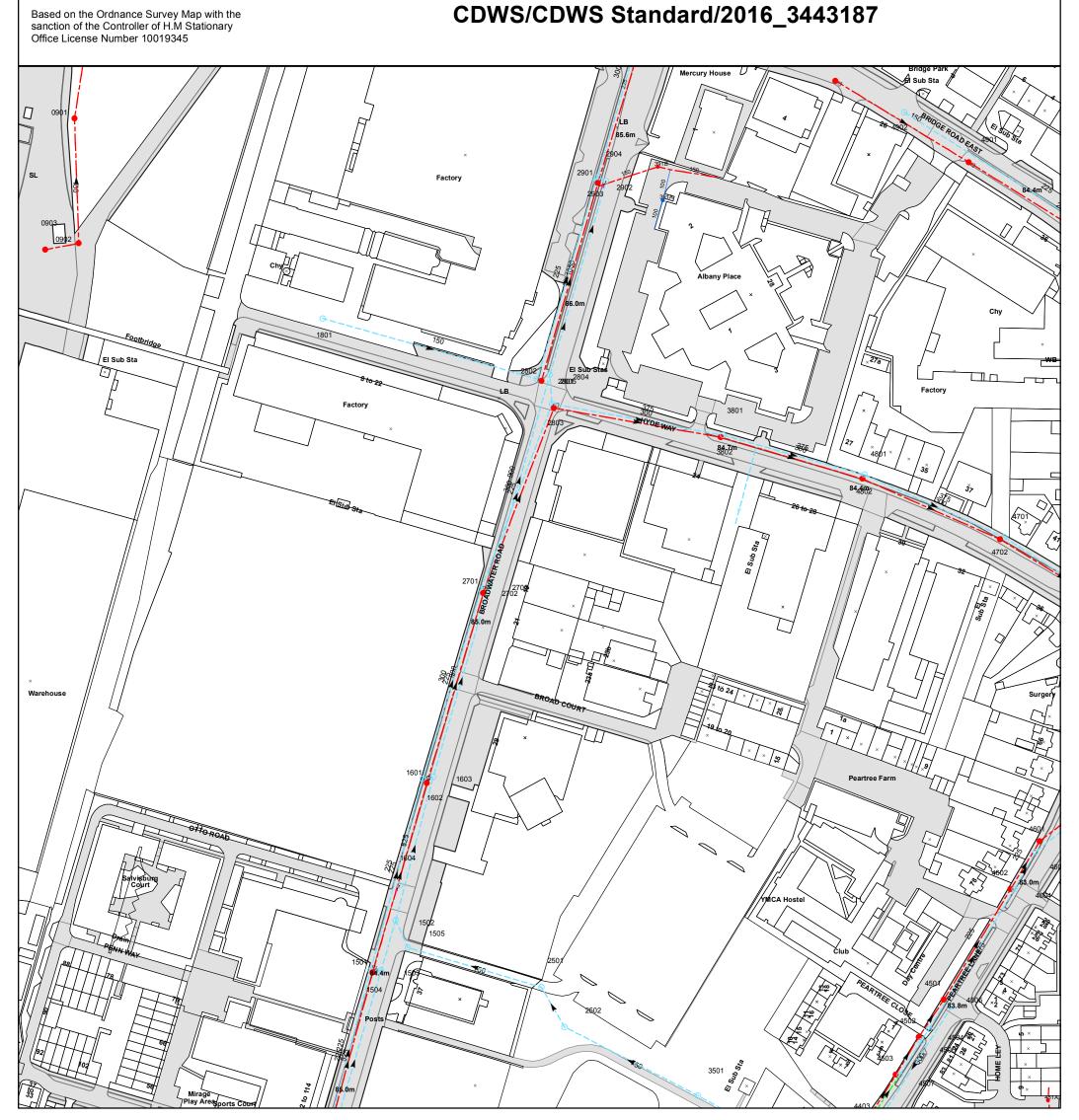


# Note

Mark-up based on topographical survey produced by Laser Surveys from October 2014 (drawing ref: L 6080 Rev1).



Appendix B – Thames Water Sewer Records





The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified before any works are undertaken. Crown copyright Reserved

Scale:	1:1790	Comments:
Width:	500m	
Printed By:	Vkumar1	
Print Date:	10/11/2016	
Map Centre:	524250,212750	
Grid Reference:	TL2412NW	

# CDWS/CDWS Standard/2016\_3443187

NB: Level quoted in metres Ordnance Newlyn Datum. The value -9999.00 indicates no Survey information is available.

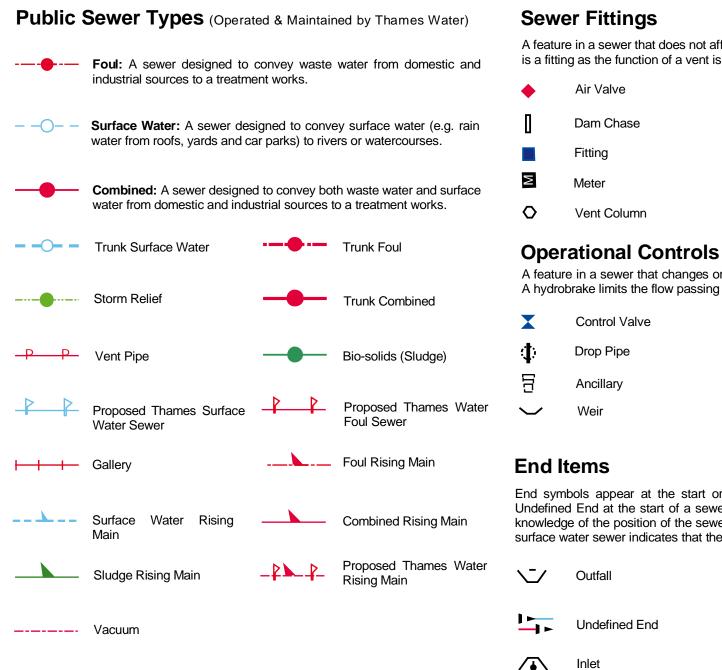
REFERENCE	COVER LEVEL	INVERT LEVEL
0903		
0902	85.57	81.33
2805	85.42	82.29
2804	85.52	78.36
2801	85.45	80.05
2903	85.77	79.65
2904	85.61	78.12
391A		
3802	84.9	79.57
3901	85.54	83
4801	84.52	79.15
4901	84.73	81.77
1504	84.59	80.58
1503	84.48	80.16
1604	84.2	83.27
1602	84.57	80.43
1603	84.5	79.65
2701	84.97	80.61
2501	83.71	80.25
4502	84.7	81.11
4504	84.51	81.02
4505	84.09	81.46
4602	83.12	79.88
4603	82.74	79.61
451A		
4503	85	81.29

REFERENCE	COVER LEVEL	INVERT LEVEL
0901	85.82	81.02
1801	85.58	83.67
2802	85.45	82.28
2803	85.47	79.98
2902	85.79	81.73
2901	85.76	81.72
391B		
3801	84.94	79.57
371A		
4702	84.04	78.76
4802	84.47	79.16
4902	85.22	83.35
1501	84.44	81.67
1502	84.29	79.96
1505	84.48	79.99
1601	84.47	81.13
2702	85.08	80.6
2703	84.97	78.92
2502	84.14	80.35
4506	84.67	81.09
4501	84.13	80.75
4701	84.06	78.69
4604	83.08	79.86
4601	82.66	79.53
3501	83.56	80.63
4507	84.97	81.3

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified before any works are undertaken. Crown copyright Reserved



# Sewer Key - Commercial Drainage and Water Enquiry



## **Sewer Fittings**

# **Other Symbols**

\*

Ø

<1

Areas

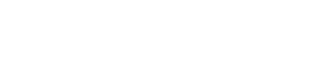
A feature in a sewer that does not affect the flow in the pipe. Example: a vent is a fitting as the function of a vent is to release excess gas.

Air Valve Dam Chase Fittina

A feature in a sewer that changes or diverts the flow in the sewer. Example: A hydrobrake limits the flow passing downstream.

	Control Valve
I	Drop Pipe
	Ancillary

End symbols appear at the start or end of a sewer pipe. Examples: an Undefined End at the start of a sewer indicates that Thames Water has no knowledge of the position of the sewer upstream of that symbol. Outfall on a surface water sewer indicates that the pipe discharges into a stream or river.





Invert Level

Agreement

Summit



### Notes:

- 1) All levels associated with the plans are to Ordnance Datum Newlyn.
- 2) All measurements on the plans are metric.
- 3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate direction of flow.
- 4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded.
- 5) 'na' or '0' on a manhole level indicates that data is unavailable.

6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in milimetres. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology present on the plan, please contact a member of Property Searches on 0118 925 1504.

Thames Water Utilities Ltd, Property Searches, PO Box 3189, Slough SL1 4W, DX 151280 Slough 13 T 0845 070 9148 E searches@thameswater.co.uk | www.thameswater-propertysearches.co.uk

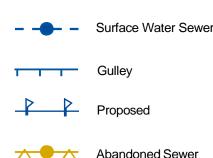
# Symbols used on maps which do not fall under other general categories

### Public/Private Pumping Station

Change of characteristic indicator (C.O.C.I.)

Lines denoting areas of underground surveys, etc.

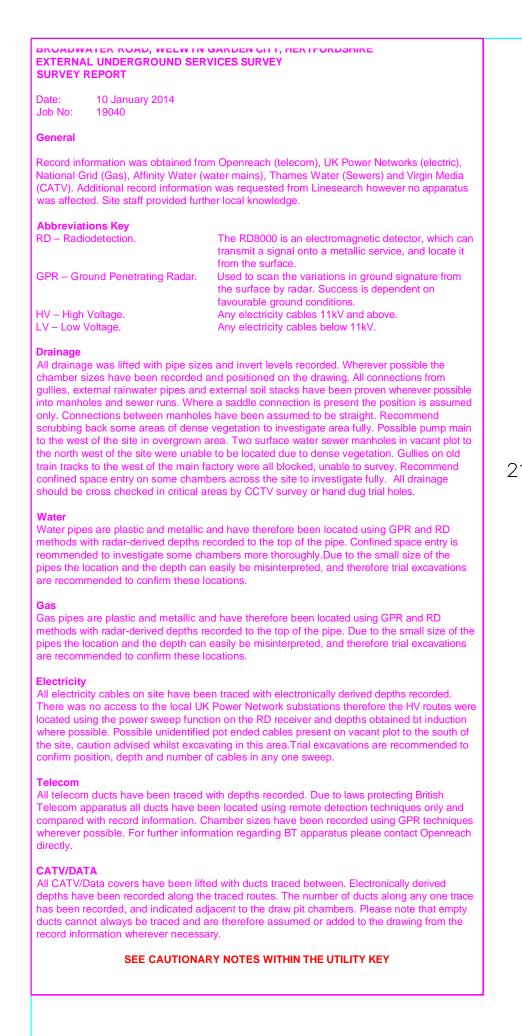
Other Sewer Types (Not Operated or Maintained by Thames Water)

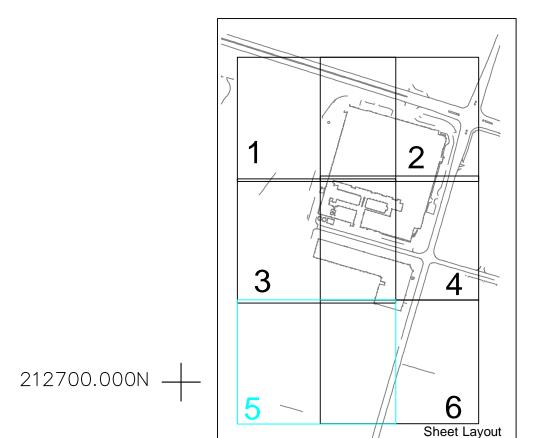


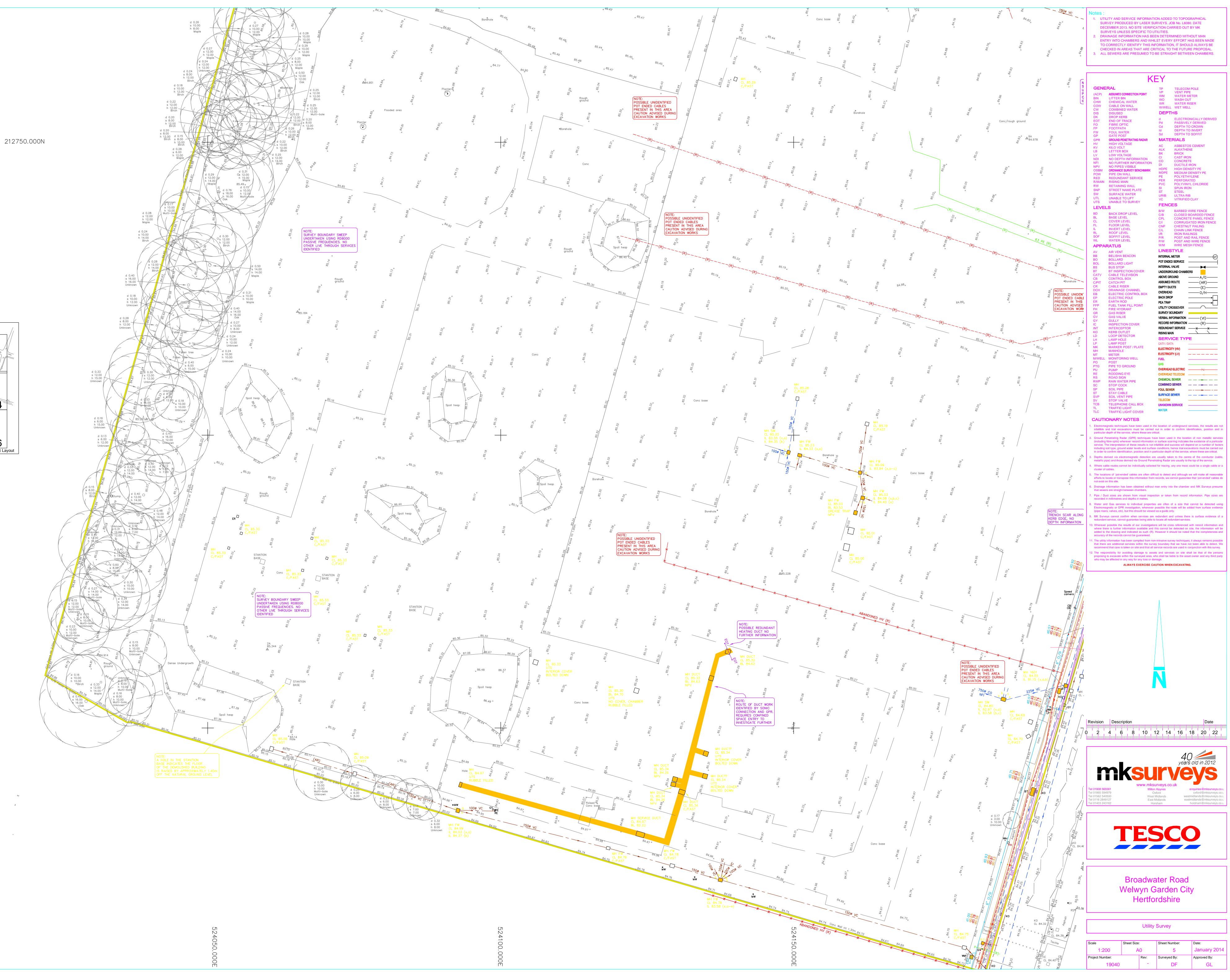


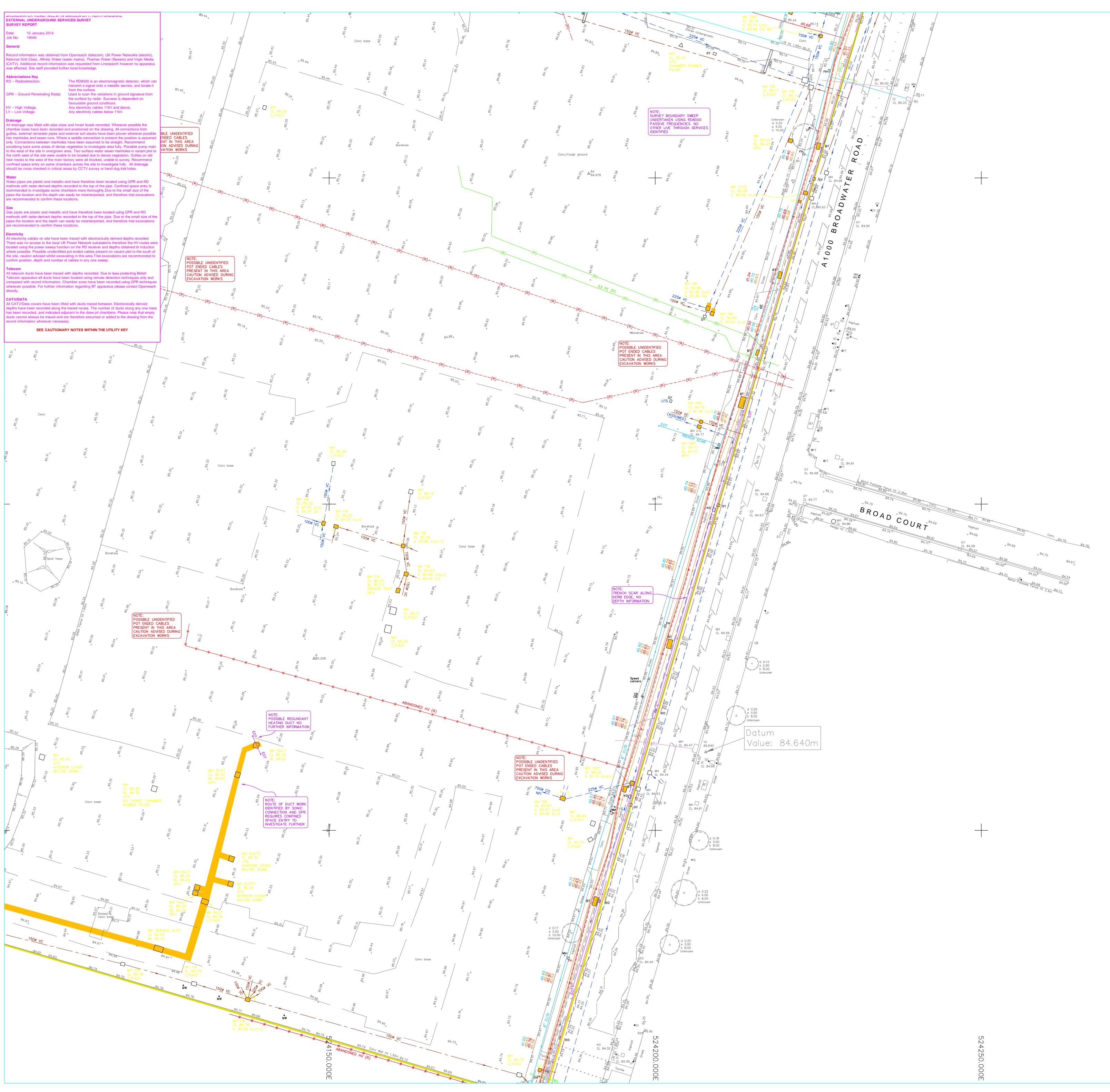


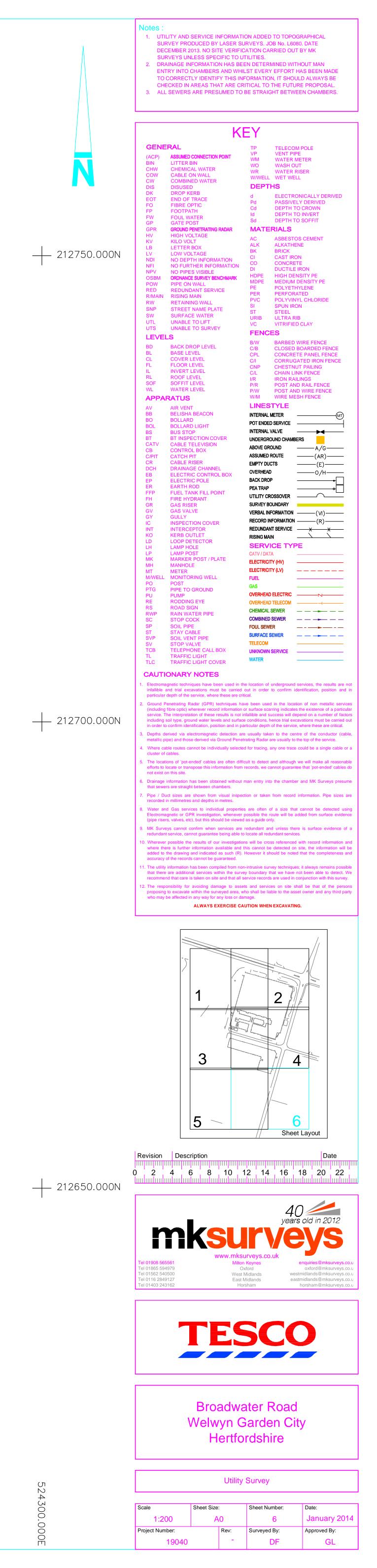
Appendix C – GPR Survey by MK Surveys





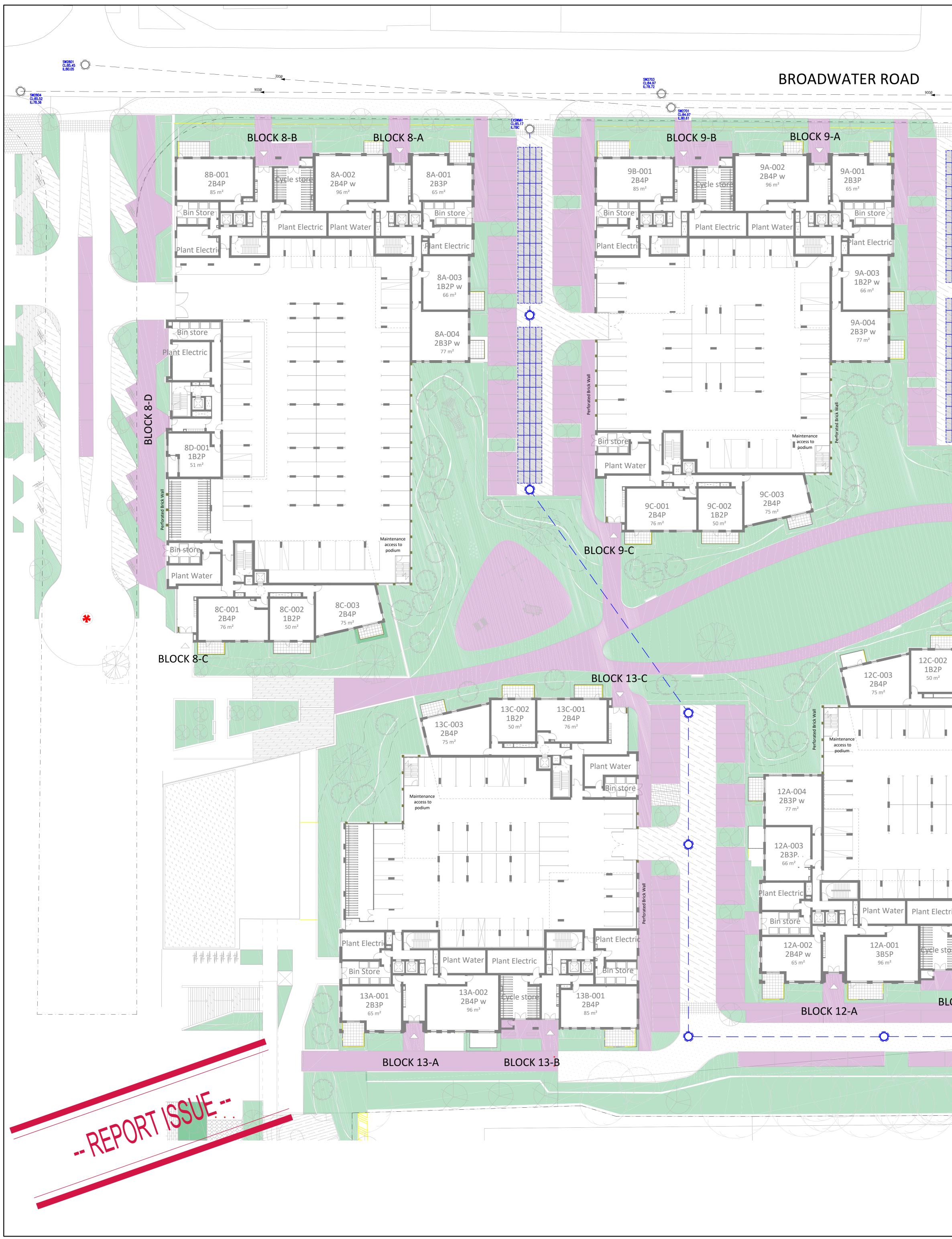




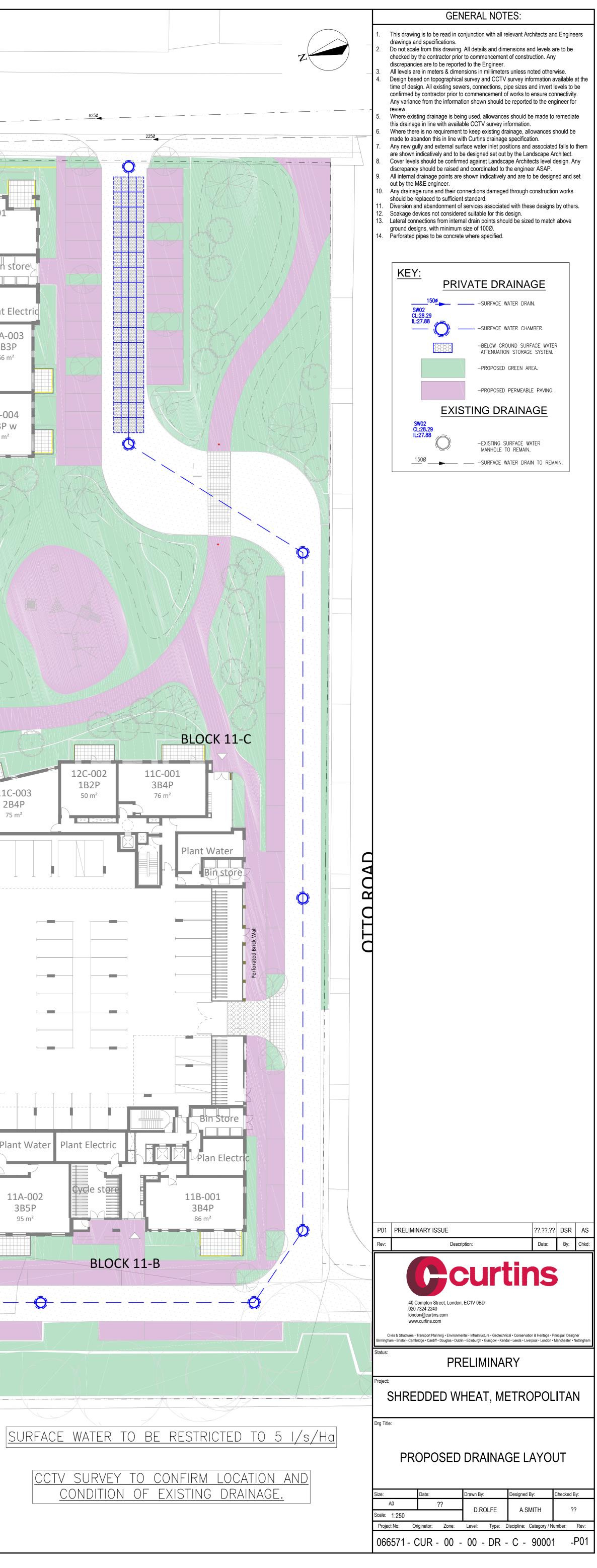




Appendix D – Proposed Drainage Layout



		SW1603 CL:84.50 IL:79.65		8250
3009 EXSWAH CL:84.77 L:81.27	BLOCK 10-B	BLOCK 10-A		
10B-001 3B4P		-002 IOA-00 2B3P		
85 m <sup>2</sup> Plant Electri		5 m <sup>2</sup>	n store	
Bin Store			nt Electric	
Perforated Brick Wall			B3P 56 m <sup>2</sup>	
		<sup>⊣</sup> 2B3	-004 P w m <sup>2</sup>	
Bin store		Maintenance		
Plant Water		Maintenance access to podium		
10C-0 3B4 76 m <sup>2</sup>	01 10C-002 P 1B2P	9C-003 2B4P 75 m <sup>2</sup>		
BLOCK 10-C				
	•			
BLOCK 12-C				2C-002 1 LB2P 50 m <sup>2</sup>
12C-001 3B4P 76 m <sup>2</sup>			75 m <sup>2</sup>	
Plant Water		Maintenance access to podium		
Binstore				
Perforated Brick Wall		11A-004 2B3P w 77 m <sup>2</sup>		
		11A-003		
Bin Store		2B3P 66 m <sup>2</sup>		
Plan Electric		Bin Store	Plant Water Plant	Electric
12B-001 3B4P 85 m <sup>2</sup>		11A-001 2B3P 65 m <sup>2</sup>	11A-002 3B5P 95 m <sup>2</sup>	cle store
СК 12-В		BLOCK 11-A		BLOCK 11-
	<u> </u>			





# Appendix E – Existing Surface Water Rates and Micro Drainage Calculations

Job Title Shreaded Wheat Factory **C**curtins Job No:066 \$11 Date: 23,02.2018 Made By: AC Checked By: Sheet No: | Existing Surface Water Runoff Estimation \* GPR\_ Survey identified existing derelict building currently discharges surface water runoff to public sewer in Brondwater Road. Rational Method Q = 2.78 g i A; = 50 mm/hr A = 1.03 Ha (Hard standing) Q = (2.78) (1) (50) (1.03) Q = 143 43

### <u>Shredded Wheat Factory – Preliminary Attenuation Estimates</u>

Runoff restricted to 5 l/s/Ha

Site area = 3.97 Ha

Runoff = (5 l/s/Ha) \* (3.97 Ha) = **19.85 l/s** 

Assuming 85% of site is impermeable:-

Impermeable area = (3.97 Ha) \* (0.85) = **3.37 Ha** 

🖌 Quick Storage	Estimate			
	Variables			
Micro Drainage	FSR Rainfall 🗸	Cv (Summer)	0.750	
brainage	Return Period (years) 100	Cv (Winter)	0.840	
Variables	Region England and Wales V	Impermeable Area (ha)	3.370	
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	19.9	
Design	Ratio R 0.419	Infiltration Coefficient (m/hr)	0.00000	
Overview 2D	-	Safety Factor	2.0	
Overview 3D		Climate Change (%)	40	
Vt				
Analyse OK Cancel Help				
Enter Area between 0.000 and 999.999				

🕖 Quick Storage	Estimate			
5	Results			
Micro Drainage	Global Variables require approximate storage of between 2012 m <sup>3</sup> and 2674 m <sup>3</sup> .			
	These values are estimates only and should not be used for design purposes.			
Variables				
Results				
Design				
Overview 2D				
Overview 3D				
Vt				
	Analyse OK Cancel Help			
	Enter Area between 0.000 and 999.999			

# **Our Locations**

### **Birmingham** 2 The Wharf

Bridge Street Birmingham B1 2JS T. 0121 643 4694 birmingham@curtins.com

### Bristol

Quayside 40-58 Hotwell Road Bristol BS8 4UQ T. 0117 302 7560 bristol@curtins.com

### Cardiff

3 Cwrt-y-Parc Earlswood Road Cardiff CF14 5GH T. 029 2068 0900 cardiff@curtins.com

### Douglas

Varley House 29-31 Duke Street Douglas Isle of Man IM1 2AZ T. 01624 624 585 douglas@curtins.com

### Edinburgh

35 Manor Place Edinburgh EH3 7DD T. 0131 225 2175 edinburgh@curtins.com

### Kendal

28 Lower Street Kendal Cumbria LA9 4DH T. 01539 724 823 kendal@curtins.com

### Leeds Woodside Mews Clayton Wood Close Leeds LS16 6QE T. 0113 274 8509 leeds@curtins.com

Liverpool Curtin House Columbus Quay Riverside Drive Liverpool L3 4DB T. 0151 726 2000 liverpool@curtins.com

### London

Units 5/6 40 Compton Street London EC1V 0BD T. 020 73242240 Iondon@curtins.com

### Manchester

10 Oxford Court Bishopsgate Manchester M2 3WQ T. 0161 236 2394 manchester@curtins.com

### Nottingham

56 The Ropewalk Nottingham NG1 5DW T. 0115 941 5551 nottingham@curtins.com

