# Salisbury Square, Old Hatfield Structural Design Certificate

Ref: 221111/DC001 Written By: P Boal

**Approved By:** B Heath **Date:** 22 November 2023

Version: A





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## Certification

Document No:	221111/DC00	221111/DC001							
	Name		Role	Signature		Date			
Prepared:	P Boal		Technical Associate			22	.11.2023		
Checked:	B Heath		Director			22.11.2023			
Revision Record									
Rev	Date	Revis	ion Notes	Ву			Check		
Α	22.11.2023	First I	ssue		РВ		ВН		



### 1.0 INTRODUCTION

- 1.1 Conisbee were instructed by Gascoyne Estates to prepare this Structural Design Certificate in relation to Welwyn Hatfield Borough Council Planning Decision Notice 6/2021/3422/MAJ, date of approval 7 December 2022. This Design Certificate is issued as part of a planning process and it should not be used for any other purpose
- 1.2 This report is intended for the use of the Client, Gascoyne Estates, and no liability can be accepted for use by any third party.

### 1.3 Planning Conditions

- 1.3.1 The planning approval decision is subject to two planning conditions relating to ground conditions and method of installing the proposed foundations conditions 5 and 8 as set out below:
- 1.3.2 Planning Condition 5 is worded as follows:

No development, other than demolition work, shall commence until a structural design certificate, completed and signed by a Chartered Engineer, and a scheme to deal with existing ground conditions has been submitted to and approved in writing by the Local Planning Authority. The Certificate shall certify that appropriate site investigations have been carried out at the site. The scheme shall include an investigation and assessment to identify those precautions or measures deemed to be required in the design and construction of the proposed development minimise any danger which might arise as a result of ground conditions.

The scheme as approved shall be fully incorporated in the design and construction of the proposed development.

REASON: To ascertain the stability of the site and to determine the structural suitability of the development thereon in view of prevailing ground conditions in accordance with the National Planning Policy Framework. To ensure that no development is undertaken which may be prejudiced by existing ground conditions in accordance with the National Planning Policy Framework.

## 1.3.3 Planning Condition 8 is worded as follows:

No development, other than demolition work, shall commence until the following has been submitted to and approved in writing by the Local Planning Authority:



- a) An Intrusive Ground Investigation to identify the current state of the site and appropriate techniques to avoid displacing any shallow contamination to a greater depth;
- b) A Risk Assessment identifying both the aquifer and the abstraction points as potential receptors of contamination; and
- c) A Method Statement detailing the depth and type of excavations (e.g. piling) to be undertaken including mitigation measures (e.g. appropriate piling design, off site monitoring boreholes etc.) to prevent and/or minimise any potential migration of pollutants to public water supply.

Thereafter, the development shall not be carried out other than in accordance with the approved details.

REASON: To protect groundwater resources, in accordance with Policy R7 of the Welwyn and Hatfield District Plan 2005; SADM18 daft Local Plan Proposed Submission August 2016; and the of the National Planning Policy Framework.

### 2.0 GROUND INVESTIGATION

- 2.1 RSK Geosciences have provided a Phase 2 Supplementary Geotechnical Investigation report 1922048 R02 (02), dated 6 October 2023. This report included in Appendix A. The report is a supplementary update report based partly on previous ground investigation reports and supplementary intrusive investigations as below:
- 2.1.1 RSK Geotechnical and Geo-Environmental Report 241882-01 (01). This included a preliminary risk assessment (PRA) followed by an intrusive site investigation undertaken 2<sup>nd</sup> to 4<sup>th</sup> February 2011. This included 2No light cable percussive boreholes (15m depth), 4No drive in window sample boreholes, and 3No shallow hand dug trial pits. Soil samples were collected and sent for geotechnical and environmental testing.
- 2.1.2 RSK Updated Geotechnical and Geo-Environmental Report 1922048 R02 (01). This report was produced to assess land contamination sources and geotechnical constraints to the proposed development based on information and data collected during the 2011 investigation, interpreted against updated guidelines.
- 2.1.3 RSK were instructed to carry out supplementary investigations works including a cable percussive borehole to 30m depth. This was carried out 4<sup>th</sup> to 16<sup>th</sup> May 2023. The objective was to confirm the geological profile below the site, including proving the depth of the natural chalk strata and level of the water table in the chalk aquifer. This information is to inform the design of deep piled foundations to the development, and the selection of an appropriate method of constructing deep piled foundations.



- 2.2 RSK Phase 2 Supplementary Geotechnical Investigation report 1922048 R02 (02) recommended that continuous flight augered piled foundations are likely to provide the most appropriate foundation option for the proposed structures.
- 2.3 RSK Phase 2 Supplementary Geotechnical Investigation report 1922048 R02 (02) recommended that if piling works are to extend deeper than 16m bgl, a Piling Works Risk Assessment may be required for the development. Pile loadings calculated by Conisbee were reviewed against typical pile working loads provided in the RSK report, and it was considered likely that piling works would need to extend beyond 16m bgl to achieve the required load resistance. Accordingly, RSK were instructed to produce the Piling Works Risk Assessment, which is referred to in section 3.0 below.

### 3.0 PILING WORKS RISK ASSESSMENT

- 3.1 RSK Geosciences have provided a Piling Works Risk Assessment (PWRA) 1922048 R03 (01), dated November 2023. This report included in Appendix B. The report refers to the prior ground investigation reports discussed in section 2.0 above.
- 3.2 The objective of the PWRA is to identify the most suitable technique to construct foundations for the proposed development, given the geology, hydrology, contamination status and constraints of the site.
- 3.3 Piled foundations are assumed to a depth of 20m, bearing into the natural chalk bedrock, with pile toes remaining above the water table of the aguifer.
- 3.4 The PWRA assesses a number of possible pollution scenarios, their applicability, and risk level in relation to continuous flight auger (CFA) bored non-displacement piling.
- 3.5 The PWRA concludes that non-displacement CFA piling is the most suitable foundation method considering the sensitive nature of the underlying controlled waters. It is noted that workmanship and groundwater levels should be monitored during construction.

### 4.0 PROPOSED FOUNDATIONS

- 4.1 Proposed structural foundation drawings for the development are included in Appendix C.
- 4.2 CFA piles are proposed as the support to all proposed foundations:



- 4.2.1 Zone 1 of the development is a 3-storey mixed used building in the middle of the site which is a reinforced concrete frame with RC columns and walls supported directly onto RC pilecaps, supported onto groups of CFA piles. The GF slab and external walls are supported onto RC ground beams spanning to the pile caps. Refer to drawings 221111-CON-01-PL-DR-S-11097 Pile Layout and 221111-CON-01-FN-DR-S-11098 Foundation Layout in Appendix C.
- 4.2.2 Zone 2 of the development is a row of five 3-storey residential townhouses at the north end of the site, to be constructed in traditional loadbearing masonry with timber floors and roof. The loadbearing walls are supported directly onto RC ground beams spanning to isolated CFA piles. Towards the eastern end of the townhouses, an existing drainage culvert passes below the footprint of the building; the ground beams here have been designed to span over the culvert and its 4.7m exclusion zone. Refer to drawings 221111-CON-02-PL-DR-S-12097 Pile Layout and Foundation 221111-CON-02-FN-DR-S-12098 Layout in Appendix C.
- 4.3 The final construction issue design of the piles will be carried out by the Contractor's specialist piling subcontractor. This will include determination of the final design depth of each pile. The piling specialist will also provide full details of the proposed piling rig, piling mat levels, and a Risk Assessment and Method Statement (RAMS) for the installation of the piled foundations on site. The piling specialist's design and RAMS will be reviewed by Conisbee for adherence to the principles set out in the Piling Works Risk Assessment prior to approval for construction.

### 5.0 CONCLUSIONS

5.1 The proposed foundation solution is continuous flight auger (CFA) bored non-displacement piles. The findings of the RSK Ground Investigation and PWRA support the use of CFA piling as the most suitable technique to construct foundations for the proposed development given the geology, hydrology, contamination status and constraints of the site.



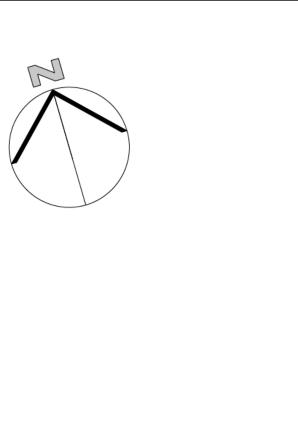
## **APPENDIX A - GROUND INVESTIGATION REPORT**



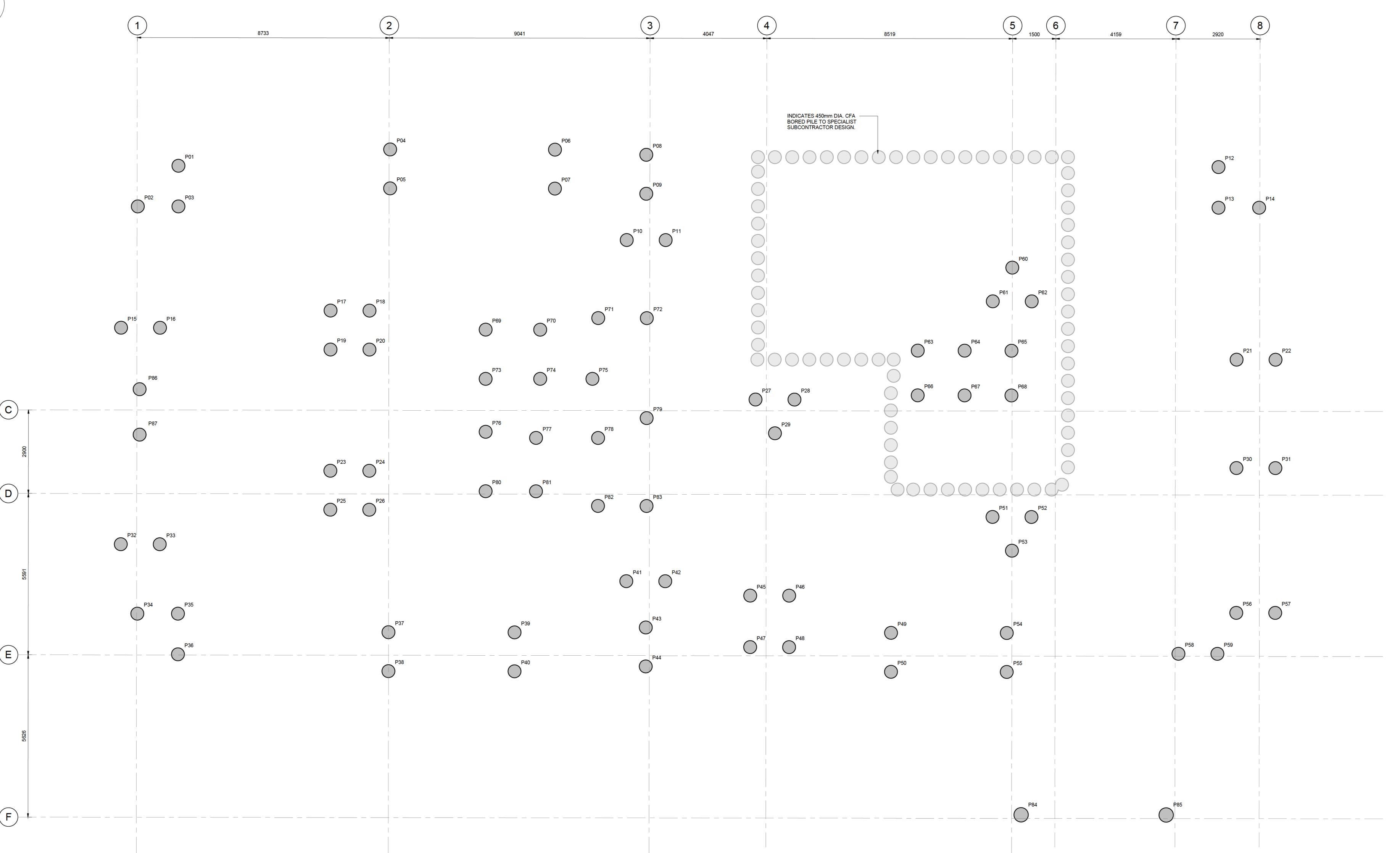
## APPENDIX B - PILING WORKS RISK ASSESSMENT



## APPENDIX C - STRUCTURAL FOUNDATION DRAWINGS



PILE LAYOUT



# GENERAL NOTES

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, ENGINEERS AND SPECIALIST DRAWINGS AND SPECIFICATIONS.

DO NOT SCALE FROM THIS DRAWING IN EITHER PAPER OR DIGITAL FORM. USE WRITTEN DIMENSIONS ONLY.

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS NOTED

ALL WATERPROOFING, DPC'S AND DPM'S TO ARCHITECTS DETAILS.

THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THE STABILITY OF THE WORKS, ADJOINING STRUCTURES AND

SERVICES AT ALL STAGES OF CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL TEMPORARY WORKS.

PROPOSED DRAINAGE SHOWN INDICATIVELY - REFER TO DRAINAGE LAYOUT DRAWING.

REFER TO DRAWING No. 22111-CON- XX-00-DR-C-2000 FOR LEVELS.

PILED FOUNDATIONS

PILED FOUNDATIONS

ASSMED 450 DIA. CFA PILES UNLESS NOTED OTHERWISE.

PILES SPACED MINIMUM 3 x PILE DIAMETER.

150mm DISTANCE AT PILE CAP OR GROUND BEAM PERIMETER.

ALL PILES TO HAVE MIN. 75mm EMBEDMENT IN PILE CAP / GROUND BEAMS.

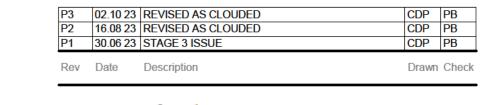
DESIGN OF PILING AND PILING MAT TO BE THE RESPONSIBIL OF THE CONTRACTOR OR THE RESPONSBILE SUB-

DESIGN OF PILING AND PILING MAT TO BE THE RESPONSIBILITY OF THE CONTRACTOR OR THE RESPONSBILE SUB-CONTRACTOR.

FOR PRELIMINARY PILE CAPACITIES REFER TO RSK SI REPORT.

PILE LENGTH IN ABEYANCE PENDING UPDATED RSK GROUND INVESTIGATION REPORT

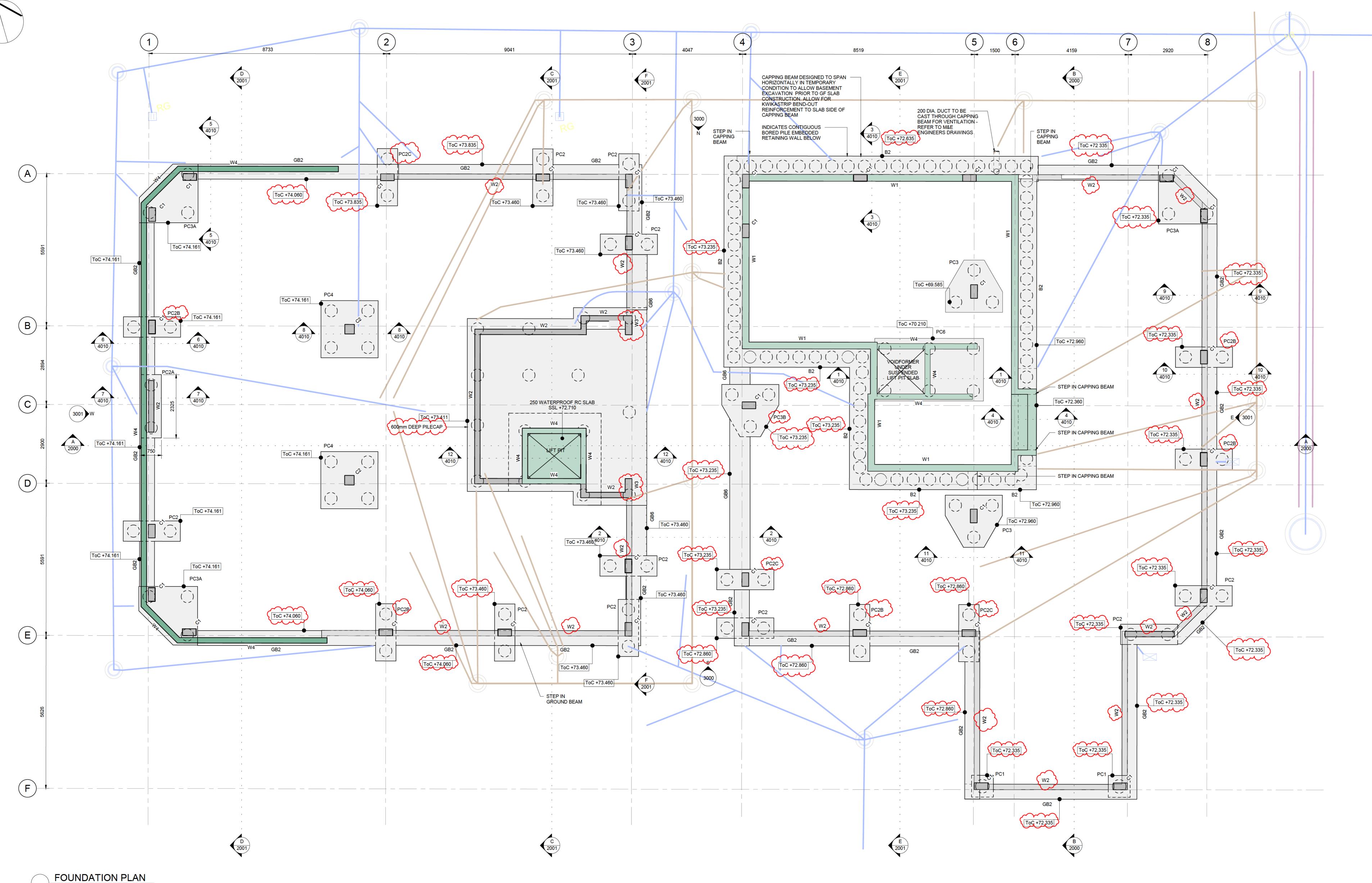




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Project SALISBURY SQUARE HATFIELD, AL9 5AD	Date
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Title	Engine
COMMERCIAL UNIT	Projec
PILE LAYOUT	221



**GENERAL NOTES** 

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DRAINAGE LAYOUT DRAWING. REFER TO DRAWING No. 22111-CON- XX-00-DR-C-2000 FOR

## FOUNDATION NOTES

FOUNDATIONS ARE TO BE CAST TO THE PROFILES INDICATED ON THE DRAWINGS. THEY ARE TO BE CAST SYMMETRICALLY ABOUT PIERS, STANCHIONS OR WALLS, UNLESS NOTED

PROVIDE COMPRESSIBLE MATERIAL OR VOID FORMER BELOW ALL GROUND BEAMS AND AGAINST INSIDE FACE OF THE EXTERNAL GROUND BEAMS WITHIN INFLUENCE ZONE OF EXISTING OR PROPOSED TREES, i.e. 155mm THICK CELLCORE ► 155 HXB 13/18 TO UNDERSIDE AND 50mm THICK HEAVE-GUARD TO INSIDE FACE OR SIMILAR APPROVED. ALLOW FOR 160mm ► THICK CELLCORE HXS GRADE 9/13 BELOW SUSPENDED GROUND 🗸 FLOOR SLAB. ALL CELLCORE TO BE PLACED ON 50mm CONCRETE BLINDING. REFER TO HEAVE PROTECTION DRAWING No. 221111-CON-01-FN-DR-S-11095 FOR EXTENT OF HEAVE PROTECTION MEASURES.

## PILES TO BE DESIGNED BY SPECIALIST TO BS EN 1997 + UK NATIONAL ANNEX.

PILES ARE TO BE DESIGNED BY SPECIALIST IN ACCORDANCE TO COMPLY WITH THE CURRENT EDITION OF 'SPECIFCATION FOR PILING AND EMBEDDED RETAINING WALLS' (SPERW), PILING

CONTRACTOR TO PAY SPECIAL ATTENTION TO THE RISK OF NEGATIVE SKIN FRICTION. CONCRETE TO BE STRENGTH GRADE C32/40 FOR WALLS, COLUMNS & SLABS; C25/30 FOR PILECAPS AND GROUND BEAMS.

CONCRETE GENERALLY: TO BS 8500-2. READY-MIXED CONCRETE: PRODUCTION PLANT: CURRENTLY CERTIFIED BY A BODY ACCREDITED BY UKAS TO BS EN 45011.

ENSURE EXCAVATIONS ARE CLEAR OF ALL DEBRIS AND ARE FREE OF WATER PRIOR TO PLACING OF CONCRETE.

FOUNDATIONS AND SUSPENDED BASEMENT / GROUND FLOOR SLABS TO BE CAST ONTO MIN. 50mm CONCRETE BLINDING.

CONCRETE TO NOT TO BE CAST AT TEMPERATURES GREATER THAN 30°C OR LESS THAN 5°C, UNLESS OTHERWISE SPECIFIED. DO NOT PLACE AGAINST FROZEN OR FROST COVERED

CONTRACTOR IS TO ENSURE THAT THE BUILDING CONTROL OFFICER IS NOTIFIED FOR THEIR INSPECTIONS PRIOR TO CONCRETING.

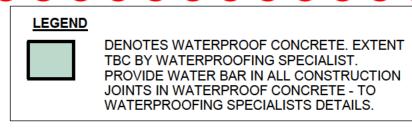
EMBEDDED RETAINING WALL NOTES

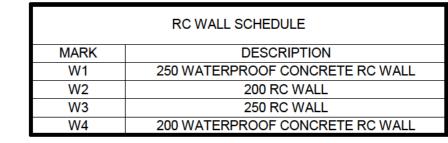
CONTINGUOUS BORED PILE RETAINING WALL TO BE DESIGNED BY SPECIALIST CONTRACTOR IN ACCORDANCE WITH STRUCTURAL SPECIFICATION SECTION D40 AND THE LATEST EDITION OF THE ICE SPERW.

RETAINING WALL IS ASSUMED TO BE PROPPED AT GF LEVEL IN THE PERMANENT CONDITION ON GRIDLINES 4, 6 AND D. RETAINING WALL IS ASSUMED TO BY FULLY CANTILEVERED

FROM BASEMENT LEVEL ALONG GRILDINE A WHERE CAPPING BEAM IS LOWERED FROM GF SLAB.

REFER TO RSK GROUND INVESTIGATION REPORT FOR SOIL PARAMETERS AND GROUNDWATER LEVELS. DESIGN FOR MINIMUM IMPOSED LOAD SURCHARGE 10kN/m2 OR 100kN ACCIDENTAL WHEEL LOAD IN PERMANENT CONDITION.





VV4	W4   200 WATERPROOF CONCRETE RC WALL			
	RC COLUMN SCHEDULE			
MARK	DESCRIPTION			
C1	(250x500 RC)			
C2	350x350 R0			
$\nearrow \nearrow \nearrow$				

GROUND BEAM SCHEDULE (COMMERCIAL) DESCRIPTION 550x450 DEEP RC GROUND BEAM 750x450 DEEP RC GROUND BEAM

PILE CAP SCHEDULE DESCRIPTION 750x750x600 DEEP RC PILECAP 2100x750x600 DEEP RC PILE CAP 2325x750x600 DEEP RC PILE CAP 2100x750x650 DEEP RC PILE CAP 2100x750x800 DEEP RC PILE CAP 2100x1919x700 DEEP RC PILE CAP 600 DEEP RC PILECAP 2100x1919x850 DEEP RC PILE CAP 2100x2100x750 DEEP RC PILE CAP

PC6	3650x2650x600 DEEP RC PILECAP
سركبر	mental
	RC BEAM SCHEDULE
MARK	DESCRIPTION
B1	250x850 DEEP RC BEAM
B2	925x600 DEEP RC CAPPING BEAM
B3	250x500 DEEP RC BEAM
B4	200x500 DEEP RC BEAM
BR1	CHS114.3x6.3

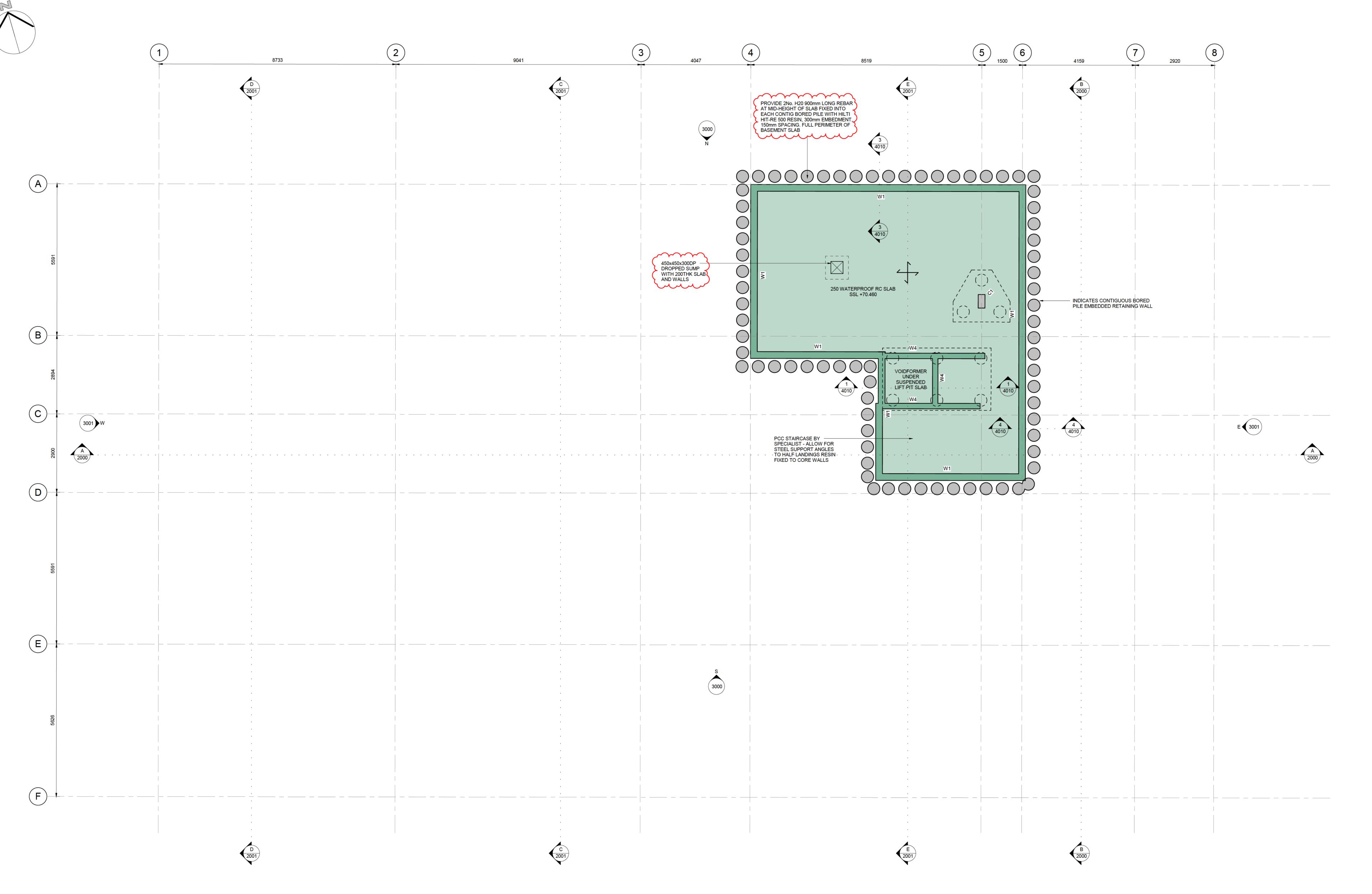
# NOT FOR CONSTRUCTION

Rev	Date	Description	Drawn	Chec
P1	23.02 23	ISSUED FOR INFORMATION	CDP	PB
P2	03.03 23	STAGE 3 ISSUE	CDP	PB
P3	24.03 23	CORES UPDATED & REVISED AS CLOUDED	CDP	PB
P4	30.06 23	UPDATED TO LATES ARCHITECTS LAYOUTS	CDP	PB
P5	16.08 23	REVISED AS CLOUDED	CDP	PB
P6	11.09 23	REVISED AS CLOUDED	CDP	PB
P7	02.10 23	REVISED AS CLOUDED	CDP	PB

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Orawing Status S4 - SUITABLE FOR STAGE A	APPROVAL
Project	Date JAN 2023
SALISBURY SQUARE HATFIELD, AL9 5AD	Scale 1:50@A0
	Drawn CDP
ïtle	Engineer PB
COMMERCIAL UNIT	Project No
FOUNDATION LAYOUT	221111
Prawing No	Revision

221111-CON-01-FN-DR-S-11098 P7



BASEMENT PLAN

# **GENERAL NOTES**

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THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL TEMPORARY WORKS.

# REFER TO DRAWING No. 22111-CON-XX-00-DR-C-2000 FOR LEVELS. CONCRETE NOTES: CONCRETE TO BE STRENGTH GRADE C32/40 FOR WALLS,

COLUMNS & SLABS; C25/30 FOR PILECAPS AND GROUND ALL RC WALLS TO BE 200mm THICK UNLESS NOTED

ALL INTERNAL PARTITIONS, & INNER LEAF OF PERIMETER CLADDING, TO BE PROPRIETARY LIGHT GAUGE STEEL SYSTEM. DESIGN OF SYSTEM & ITS FIXINGS TO RC FRAME BY THE SYSTEM SUPPLIER.

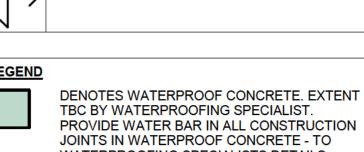
ALL BALUSTRADE FIXINGS TO RC FRAME BY THE SYSTEM

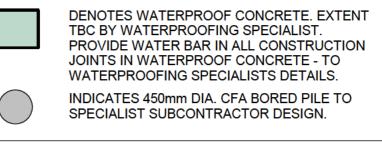
# ALL RC COLUMNS SET OUT TO CENTRELINE & ON GRID

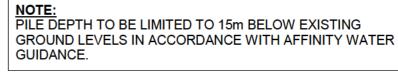
UNLESS NOTED/DIMENSIONED OTHERWISE.

NOTE:
M&E SERVICES VOIDS SHOWN INDICATIVELY - TBC BY M&E

FLOOR CONSTRUCTION LEGEND						
SYMBOL	DESCRIPTION					
	INDICATES 2-WAY SPANNING RC FLAT SLAB					

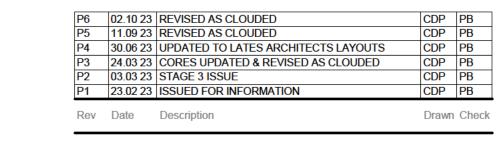






MADIC	RC WALL SCHEDULE
MARK	DESCRIPTION OF CONCRETE DO WAL
W1	250 WATERPROOF CONCRETE RC WAL
W2	200 RC WALL
W3	250 RC WALL
W4	200 WATERPROOF CONCRETE RC WAL
MARK	DESCRIPTION
C1	250x500 RC
C2	350x350_RC
,	<del>~~~~~~~~~</del>
ALLOW FOR 1	60DP CELLCORE GRADE 9/13 BELOW BASEN
SLAB. ON MIN	50mm CONCRETE BLINDING. REFER TO HE
	DRAWING No. 221111-CON-01-FN-DR-S-1109
,	DRAWING NO. 221111-CON-01-FN-DR-3-1103

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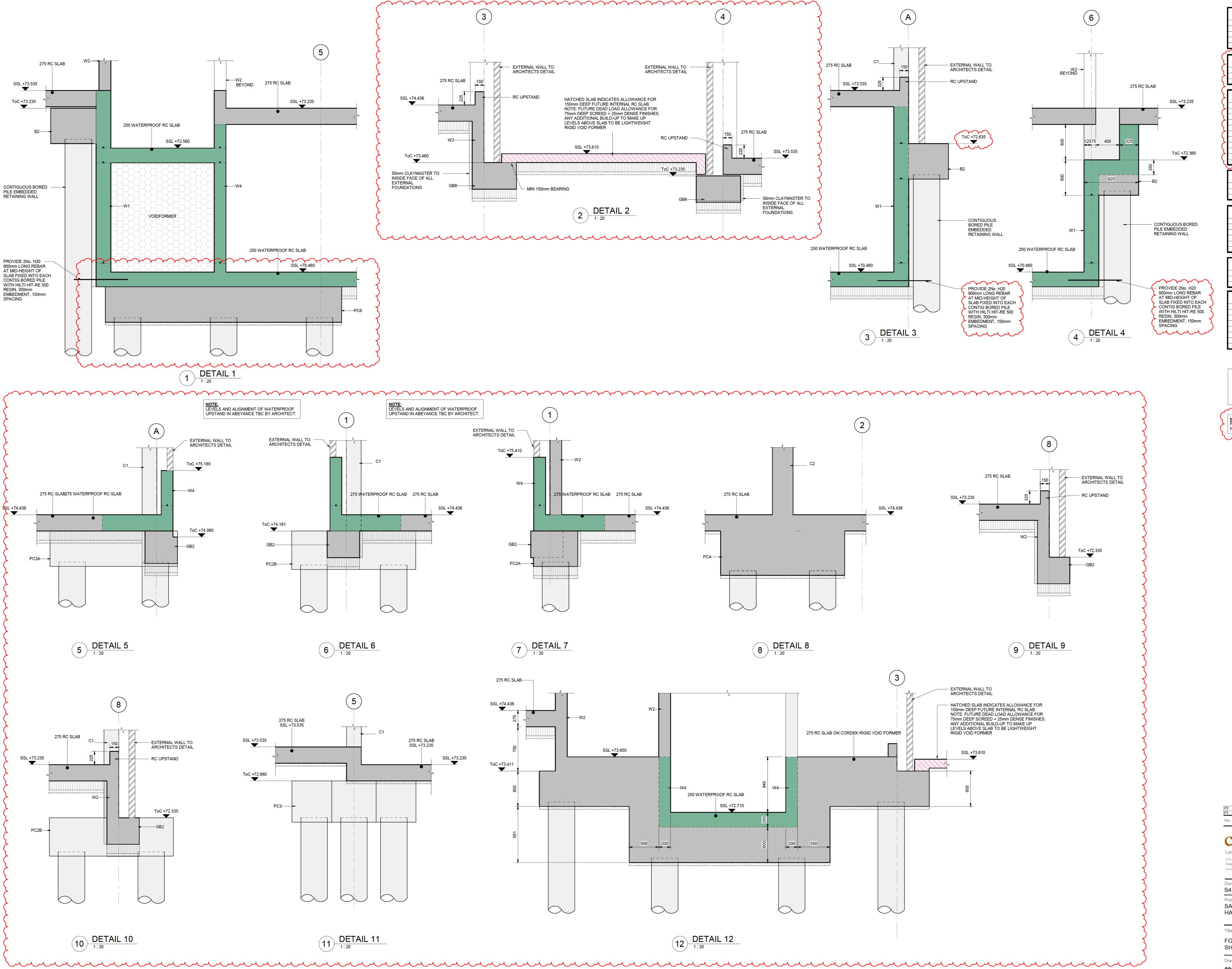
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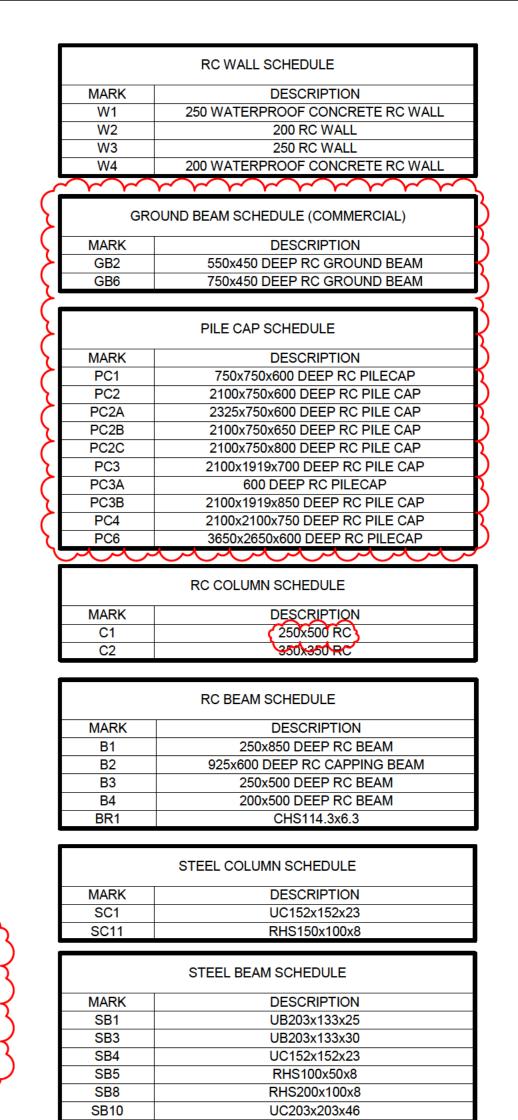
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Project	Date JAN 2023
SALISBURY SQUARE HATFIELD, AL9 5AD	Scale 1:50@A0
	Drawn CDF
Title	Engineer PE

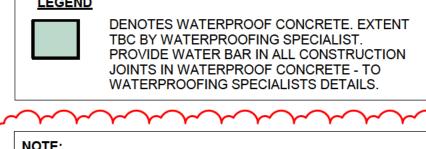
Project No

COMMERCIAL UNIT BASEMENT LAYOUT 221111

Drawing No Revision 221111-CON-01-B1-DR-S-11099







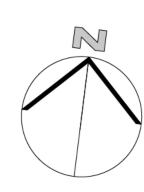
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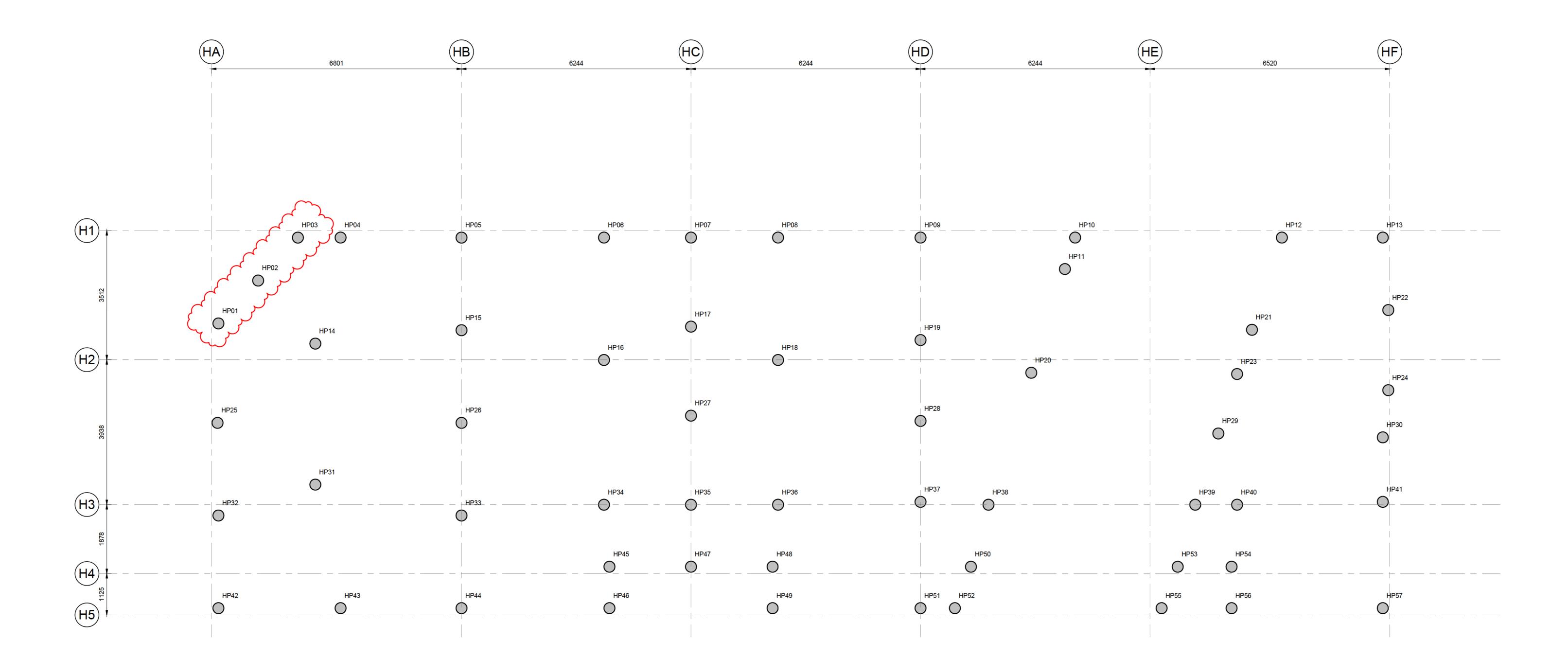
NOTE:
FOR EXTENT OF HEAVE PROTECTION REFER TO DRAWING
No. 221111-CON-01-FN-DR-S-11095

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HATFIELD, AL9 5AD	SAsaindicat	ed@	
,	Drawn	CI	
Title	Engineer		
FOUNDATION DETAILS	Project No	)	
SHEET 1	221111		
Drawing No	Revision		
221111-CON-XX-XX-DR-S-4010	P2		





PILE LAYOUT

# **GENERAL NOTES**

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OTHERWISE.

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REFER TO DRAWING No. 22111-CON- XX-00-DR-C-2000 FOR LEVELS.

PILED FOUNDATIONS

CONTRACTOR.

ASSMED 300 DIA. CFA PILES UNLESS NOTED OTHERWISE.

PILES SPACED MINIMUM 3 x PILE DIAMETER.

150mm DISTANCE AT PILE CAP OR GROUND BEAM PERIMETER.

GROUND BEAMS.

DESIGN OF PILING AND PILING MAT TO BE THE RESPONSIBILITY
OF THE CONTRACTOR OR THE RESPONSBILE SUB-

ALL PILES TO HAVE MIN. 75mm EMBEDMENT IN PILE CAP /

FOR PRELIMINARY PILE CAPACITIES REFER TO RSK SI REPORT.

PILE LENGTH IN ABEYANCE PENDING UPDATED RSK GROUND INVESTIGATION REPORT

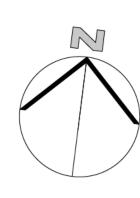
# NOT FOR CONSTRUCTION

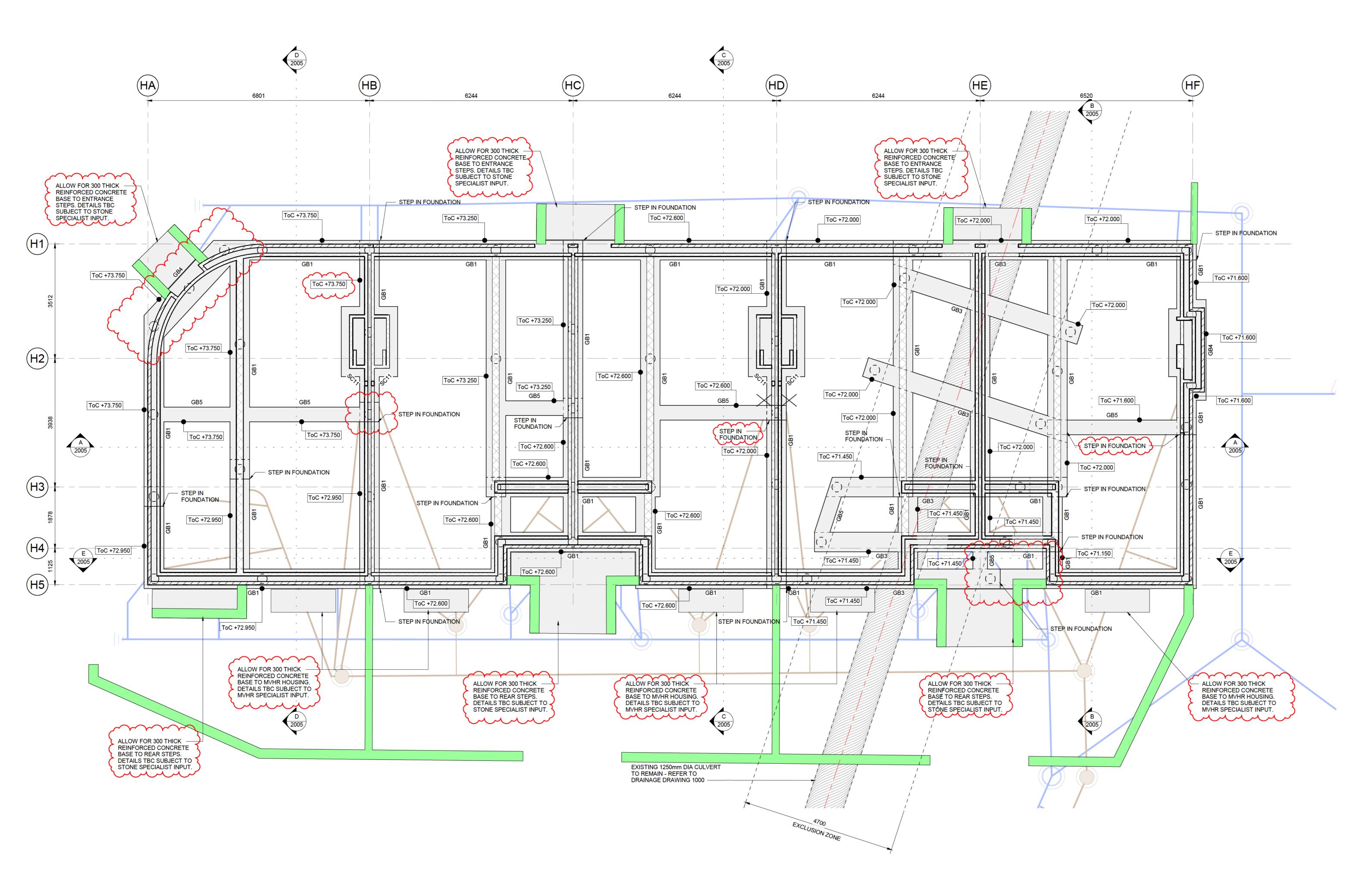
P1           30.06 23   STAGE 3 ISSUE           CDP   PB             Rev         Date         Description         Drawn Check	P2	02.10 23	REVISED AS CLOUDED	CDP	PB
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Project	Date JAN Scale 1:50	
SALISBURY SQUARE HATFIELD, AL9 5AD		
,	Drawn	
Title	Engineer	
HOUSES	Project No <b>221111</b>	
PILE LAYOUT		
Drawing No	Revision	
221111-CON-02-PL-DR-S-12097	P2	





FOUNDATION PLAN

GROUND BEAM SCHEDULE (HOUSES) DESCRIPTION 600x450 DEEP RC GROUND BEAM 600x750 DEEP RC GROUND BEAM

INDICATES 300mm DIA. CFA BORED PILE -75mm EMBEDMENT INTO GROUND BEAMS.

INDICATES STEPOC REINFORCED BLOCKWORK RETAINING WALL WITH BRICK FACING AND METAL RAILING BY SPECIALIST -REFER TO TYPICAL RETAINING WALL DETAILS ON DRG No. 221111-CON-XX-XX-DR-S-4050

900x450 DEEP RC GROUND BEAM 450x450 DEEP RC GROUND BEAM

ALL WATERPROOFING, DPC'S AND DPM'S TO ARCHITECTS

DRAWINGS AND SPECIFICATIONS.

**GENERAL NOTES** 

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS NOTED

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL

DO NOT SCALE FROM THIS DRAWING IN EITHER PAPER OR

RELEVANT ARCHITECTS, ENGINEERS AND SPECIALIST

DIGITAL FORM. USE WRITTEN DIMENSIONS ONLY.

THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THE STABILITY OF THE WORKS, ADJOINING STRUCTURES AND SERVICES AT ALL STAGES OF CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND

INSTALLATION OF ALL TEMPORARY WORKS. REFER TO DRAWING No. 22111-CON-XX-00-DR-C-2000 FOR

EXTERNAL LEVELS. PROPOSED DRAINAGE SHOWN INDICATIVELY - REFER TO

# FOUNDATION NOTES

FOUNDATIONS ARE TO BE CAST TO THE PROFILES INDICATED ON THE DRAWINGS. THEY ARE TO BE CAST SYMMETRICALLY ABOUT PIERS, STANCHIONS OR WALLS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.

PROVIDE COMPRESSIBLE MATERIAL OR VOID FORMER BELOW ALL GROUND BEAMS AND AGAINST INSIDE FACE OF THE EXTERNAL GROUND BEAMS WITHIN INFLUENCE ZONE OF EXISTING OR PROPOSED TREES, i.e. 155mm THICK CELLCORE 155 HXB 13/18 TO UNDERSIDE AND 50mm THICK HEAVE-GUARD TO INSIDE FACE OR SIMILAR APPROVED. REFER TO HEAVE PROTECTION DRAWING 22111-CON-02-FN-DR-S-12095 FOR EXTENT OF HEAVE PROTECTION MEASURES. ALL CELLCORE TO

BE PLACED ON 50mm CONCRETE BLINDING. PILES TO BE DESIGNED BY SPECIALIST TO BS EN 1997 + UK NATIONAL ANNEX. PILES ARE TO BE DESIGNED BY SPECIALIST IN ACCORDANCE TO

PILING AND EMBEDDED RETAINING WALLS' (SPERW), PILING CONTRACTOR TO PAY SPECIAL ATTENTION TO THE RISK OF NEGATIVE SKIN FRICTION. CONCRETE TO BE STRENGTH GRADE C25/30 THROUGHOUT.

COMPLY WITH THE CURRENT EDITION OF 'SPECIFCATION FOR

CONCRETE GENERALLY: TO BS 8500-2. READY-MIXED CONCRETE: PRODUCTION PLANT: CURRENTLY CERTIFIED BY A

BODY ACCREDITED BY UKAS TO BS EN 45011. ENSURE EXCAVATIONS ARE CLEAR OF ALL DEBRIS AND ARE

FREE OF WATER PRIOR TO PLACING OF CONCRETE. FOUNDATIONS AND SUSPENDED BASEMENT / GROUND FLOOR SLABS TO BE CAST ONTO MIN. 50mm CONCRETE BLINDING.

CONCRETE TO NOT TO BE CAST AT TEMPERATURES GREATER THAN 30°C OR LESS THAN 5°C, UNLESS OTHERWISE SPECIFIED. DO NOT PLACE AGAINST FROZEN OR FROST COVERED SURFACES.

CONTRACTOR IS TO ENSURE THAT THE BUILDING CONTROL OFFICER IS NOTIFIED FOR THEIR INSPECTIONS PRIOR TO

SUBSTRUCTURE MASONRY AND PRE CAST FLOORING

WORKMANSHIP IS TO COMPLY GENERALLY WITH BS 5628 PARTS 1 & 3. BRICKWORK TO BE BS 3921. BLOCKWORK TO BE TO BS BRICKWORK/BLOCKWORK BELOW DPC IS TO BE MINIMUM FL

DURABILITY. MORTAR TO BE M12 (i), SRPC, UNLESS NOTED OTHERWISE ON THE DRAWINGS. DO NOT LAY MASONRY WHEN THE AMBIENT AIR TEMPERATURE

IS LESS THAN 5°C. BLOCKWORK TO BE CELCON HIGH STRENGTH MIN. 7.3N/mm<sup>2</sup> OR SIMILAR APPROVED.

MINIMUM SUB FLOOR VOID OF 250mm (AS RECOMMENDED BY NHBC GUIDANCE).

THE PRE-CAST MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN, SUPPLY AND ERECTION OF ALL PRECAST CONCRETE ELEMENTS. THE MANUFACTURER SHALL REQUEST ADDITIONAL INFORMATION AS NECESSARY FROM THE C.A., LIAISING WITH THE C.A., CONTRACTOR AND OTHERS AS NECESSARY TO ENSURE CO-ORDINATION OF THE WORK WITH RELATED BUILDING ELEMENTS AND SERVICES. THE CONTRACTOR IS TO ALLOW FOR ALL DETAILS/WORKMANSHIP

FABRICATION DRAWINGS TO BE PREPARED WELL IN ADVANCE TO ALLOW FOR SUBMISSION AND COMMENTS TO BE MADE BY THE DESIGN/CLIENT TEAM. NO FABRICATION WORK SHALL COMMENCE UNTIL ALL COMMENTS HAVE BEEN RECEIVED AND ANY NECESSARY AMENDMENTS INCORPORATED.

REQUIRED BY THE MANUFACTURER'S TYPICAL DETAILS.

# FLOOR LOADINGS:

DL - 1.8kN/m² (ASSUMING INSULATION, SCREED AND FINISHES) LL - 1.5kN/m<sup>2</sup> + 1.0 kN/m<sup>2</sup> FOR PARTITIONS AND 2.0kN POINT LOAD 

**CONTROL OF BACKFILL BELOW HOUSES:** 

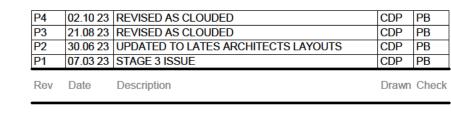
BACKFILL IS TO BE PLACED EVENLY IN LAYERS BOTH SIDES OF GROUND BEAMS AND MASONRY WALLS. AT NO POINT SHOULD BACKFILL LEVELS DIFFER BY MORE THAN 450mm ACROSS WALLS IN THE TEMPORARY OR PERMANENT CONDITION.

GEN 3 CONCRETE FILL IS TO BE PROVIDED IN ALL CAVITY WALLS UP TO PROPOSED GROUND/BACKFILL LEVEL AND CURED IN ADVANCE OF BACKFILLING ACTIVITIES - CONTRACTOR TO CONFIRM AGAINST ARCHITECTS CAVITY DETAILS.

BACKFILL GRADIENTS TO BE NO GREATER THEN 1:3.

ALL BACKFILL LEVELS TO BE AGREED WITH THE ENGINEER IN ADVANCE OF PLACING,

# NOT FOR CONSTRUCTION



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